GENERAL NOTES

- 1. These drawings have been prepared primarily to safeguard against major structural damage and loss of life, not to limit damage or maintain function-as per requirements of the current accepted building code as listed in the Basis for Design.
- 2. An experienced licensed contractor with a working knowledge of applicable codes and industry accepted standard practices shall perform the work depicted in these drawlings.
- 3. All work shall conform to the minimum standards of the current governing building code and industry specific specifications and standards. The contractor shall comply with requirements of all regulatory agencies with authority over any portion of the work. Work not explicitly shown on these drawings shall conform to all applicable codes and accepted standard practices.
- 4. The contractor shall verify all dimensions, elevations and conditions on these drawings with the architectural and all other discipline drawings prior to start of construction. Notify architect or EOR in writing before the start of construction regarding discrepancies, omissions or variations, or they shall become the sole responsibility of the contractor. Notes and the specific details on these drawings take precedence over general structural notes and typical details.
- 5. Construction methods are not indicated on these drawings. The contractor shall be solely responsible for all methods, sequences and procedures of construction. The contractor shall provide adequate shoring, bracing, formwork, etc. as required for the protection of life and property during construction.
- 6. Openings, pockets, etc. larger than 6 inches shall not be placed in structural members unless specifically detailed on these drawings. When drawings by others show items in structural members not shown on the structural drawings, notify the engineer in writing to determine correct disposition.
- 7. Site visits by the EOR are a resource for the contractor and shall not be considered as special inspections.
- 8. General inspection and special inspection shall be performed during construction as required by the local governing municipality.

BASIS FOR DESIGN

- 1, Governing Building Code: 2006 International Building Code
- 2. Occupancy Category: II
- 3. Roof Loads:

Dead Load = self weight + 0.25 psf misc, dead load

Live Load = 5 psf (non-reducible)

Snow Load = 25 psf (minimum required snow load)

4. Wind Design:

Basic Wind Speed = 85 mph Wind Exposure = D

Wind Importance Factor = 1.0 (Category II) Mean Roof Helght = less than 18'-0"

5. Seismic Design:

Wind loads govern lateral structural design.

CONCRETE

1. Foundation design is based upon fic = 2500 PSI for all concrete in contact with the soil. Geotechnical Engineer shall provide information regarding on-site soils with sulfate exposures as per current accepted code as listed in the basis for design or as follows:

| Exposure | w/c | 28-day (f'c) | Sol. Sulfate | Cemen |
|-------------|------|--------------------|--------------|-------|
| Moderate | 0.50 | 4000 PSI | 0.1 - 0.2 II | |
| Severe | 0.45 | 4500 PSI 0.2 - 2.0 | V | |
| Very severe | 0.45 | 4500 PSI over 2.0 | V + pozzolan | |

Minimum 28-day strength concrete placed in the field shall be (fc), 3000 PSI.

- 2. Concrete mixes shall be designed by a certified concrete testing laboratory and approved by the EOR.
- 3. All concrete shall be normal weight 145 PCF with hardrock aggregates.
- 4. Maximum slump shall be 5 inches, and the water shall be clean and potable.
- 5, Portland cement shall be ASTM C 150 type V for concrete in contact with soil. Type II cement may be used elsewhere. Type V plus pozzolan shall be used where concrete is in contact with soll containing very severe sulfate exposure.
- 6. Fly ash shall comply with ASTM C 618, class F, and shall be approved by the Architect in writing before being used on the job. When used, fly ash content shall be 15 percent to 25 percent. Water-cement ratio shall be based on total cementitous material.
- 7. Aggregates shall comply with ASTM C 33, use 3/4" maximum aggregate size in structural concrete, 1 1/2" maximum in slabs on grade and 3/8" pea gravel in grouts only unless specifically noted otherwise on the plans or by approval by the EOR in writing.
- 8. High early strength concrete may be used when requested by the contractor. Mix design data using field-cured specimens shall be submitted for review and approval.
- 9. No more than 90 minutes shall elapse between concrete batching and placement unless approved by the EOR in writing.
- 10. Concrete mixing, placement and quality shall be per the current accepted code as listed in the basis for design. Mechanically vibrate all concrete. Vibrate slabs on grade around and under floor ducts or similar elements.
- 11. Remove all debris from forms before placing concrete. Concrete shall be carefully placed in reinforced elements so as to avoid segregation of aggregates. Unconfined fall of concrete shall not exceed five feet unless approved the by EOR in writing.
- 12. Reinforcing, dowels, bolts, anchors, sleeves, embeds, etc shall be securely positioned in the forms prior to placement of the concrete.
- 13. Protect concrete from damage or reduced strength due to cold or hot weather in accordance with ACI 305 and 306. Contractor shall take special curing precautions to minimize shrinkage cracking of concrete slabs.
- 14. Pipes and electrical condults shall not be embedded in structural concrete except where specifically approved by the EOR. Embedded items shall not impair the strength of the

STRUCTURAL STEEL

1. Structural steel members shall conform to the following standards and material properties, U.N.O.:

ASTM A36 (36 ksl) Standard steel shapes: ASTM A992 (50 ksl) Rolled wide flange sections: ASTM A36 (36 ksl) Bars and plates: ASTM A500 Grade B (46/42 ksi) Tubes and pipes: ASTM A53 Grade B (35 ksl) Standard pipes: ASTM A653 (50 ksi) Cold formed tubes, studs, etc.:

- High strength bolts: ASTM A325 All other bolts:
- 2. Where steel beam is used in connection to wood framing, a 3x DF-L stud grade plate will be boilted to the top flange with 1/2" dla. boilts staggered at 24" o.c.
- 3. Structural steel shall be fabricated and erected in accordance with AISC specifications or the design fabrication and erection of structural steel buildings.
- 4. All welds shall be minimum 1/8" E70XX all around weld, U.N.O.
- 5, Welders shall be AWS certified or equivalent (WAVO). All welding shall use E70 series low hydrogen electrodes. All welding shall conform to the latest American Welding Society standards; welds on drawings are shown as shop welds. Contractor may shop weld or field weld at his discretion. All full penetration welds shall be tested and certified by an independent testing laboratory.
- 6. All bolts shall be installed as bearing-type connections with threads excluded from shear plane (type "x" connection), U.N.O. High-strength bolts shall be snug tightened using any AISC approved method and do not require special inspections unless noted otherwise. All bolts in slotted or oversize holes and all high-strength bolts shall be
- 7. All expansion or epoxy bolts shall have current ICBO/ICC rating for material into which installation occurs. Headed stude shall conform to all requirements of the latest edition of the "recommended practices for stud welding" and the "structural welding code" published by AWS. All bolts, anchor bolts, expansion bolts, etc. shall be installed with steel washers at face of wood.

- 8. Grout beneath column bases or bearing plates shall be 5000 psi minimum non-shrink flowable grout or drypack. Install grout under bearing plates before framing members are installed. At columns, install grout under base plates after column has been plumbed but prior to floor or roof installation. Grout depth shall be sufficient to allow grout or drypack to be placed beneath plate without voids.
- 9. Welded moment frame connections shall comply with the following requirements:

Access holes shall be smooth and free of notches.

Minimum preheat per AWS requirements shall be maintained for all welds including tack welds. Verify preheat with temperature indicating crayons,

Weld material shall comply with Charpy V-Notch requirement of 20 ft-lbs at -20° F.

End dams are not allowed.

All weld layers shall be completed full width of flange before new layer is started.

Maximum layer thickness equals 1/4", maximum bead width equals 5/8".

Grind edges of weld smooth and flush with edges of abutting parts.

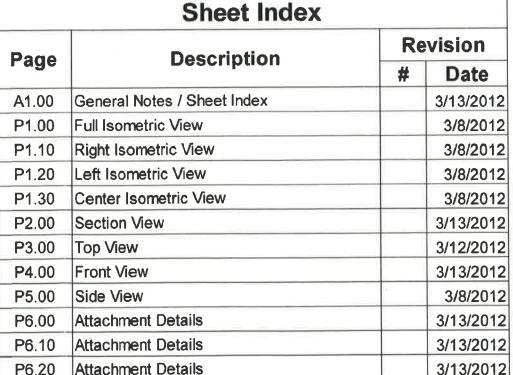
Notches or grooves shall be filled and ground smooth.

10. All misc, welds not noted, including stiffeners, misc, plates, etc. shall be per AISC manual table J2.4.

- 1. Screws shall be self-tapping hex head sheet metal screws with current ICBO/ICC rating for material into which Installation occurs.
- 2. Screws which are removed shall be replaced with a larger diameter screw where replacement is made into an existing hole. Replace all screws which strip out material.
- 3. Screws shall be spaced no closer than 5/8" o.c. and maintain a minimum free edge distance of 1/2", U.N.O.
- 4. Clip angles or flat clips used for attachment shall be a minimum of 33 mils, U.N.O. Size all clip angles and flat clips to maintain minimum screw spacing and edge distances as noted above.
- 5. All screws #8 and larger shall have a minimum head size of 5/16". All screws shall have sufficient length to ensure penetration into aluminum/steel by at least (2) full diameter
- 6. All screws shall be a minimum of #8 self-tapping screws, U.N.O.
- 7. All anchors installed into concrete shall be Hilli KB TZ expansion anchors (ESR-1917) or Hilli HIT-RE 500-SD adhesive anchors (ESR-2322). All anchors installed into CMU shall be Hillt KB III expansion anchors (ESR-1385) or Hillt HIT HY-150 adhesive anchors (ESR-1967). Approved equivalent anchors with current ICBO/ICC reports may be used at the contractor's discretion.
- 8. All bolts shall be installed as bearing-type connections with threads excluded from shear plane (type "x" connection), U.N.O. High-strength bolts shall be snug tightened using any AISC approved method and do not require special inspections unless noted otherwise. All bolts in slotted or oversize holes and all high-strength bolts shall be installed with washers.

SPECIAL INSPECTION

- 1. All required special inspections shall be performed in accordance with the current accepted building code. Check with the local building department for required special inspections.
- 2. The special inspector shall be a qualified person who shall demonstrate competence, to the satisfaction of the local building department, for the inspection of the particular type of construction or operation requiring special inspection.
- 3. The special inspector shall inspect the work assigned for conformance with the approved contract drawings and specifications. The special inspector shall furnish inspection reports to the local building department, the EOR and other designated persons required by the governing building official. All discrepancies shall be brought to the immediate attention of the contractor for correction, and then if uncorrected in a reasonable period of time, the EOR and the building official shall be notified. The special inspector shall submit a final signed report stating whether the work requiring special inspection was, to the best of the inspectors knowledge, constructed in conformance with the approved plans and specifications and the applicable building code provisions
- 4. Inspectors shall inspect from an approved set of contract drawings, shop drawings shall not be used in lieu of the approved contract drawings for Inspection purposes.
- 5. Certificate of approval regarding materials and inspection of prefabricated items shall be provided in accordance with the current accepted building code.



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CARILLON **PROPERTIES** 3240 CARILLON PT KIRKLAND, WA 98033

WILLARD PARK

LOADING DOCK REPAIR

ENG. NOTES

161627 RI Work Order Number

IAN M Drawn B

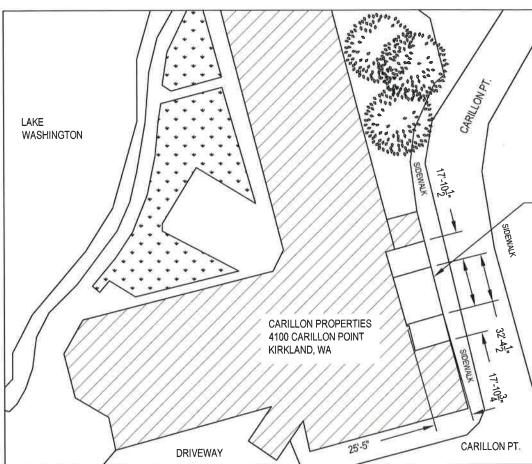
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MICHAEL BABBITT

REPLACING DAMAGED CANOPY, ONLY CENTER SECTION

NOTICE

HOURS OF WORK: 7AM TO 8PM MON-FRI 9AM TO 6PM SAT. NO WORK SUNDAYS & HOLIDAYS (PER KZC SEC. 115.25) Exceptions must be approved in writing by Planning Official



REPLACEMENT CANOPY





| | Sneet Index | | |
|-------|-------------------------------------|----|-----------|
| Page | Description | Re | evision |
| raye | Description | # | Date |
| A1.00 | Engineering Notes | | 3/13/2012 |
| A1.10 | Aerial Photo / Sheet Index | | 6/21/2012 |
| A1.20 | East Elevation View | | 6/21/2012 |
| P1.00 | Full Isometric View / Scope of Work | | 3/8/2012 |
| P1.10 | Right Isometric View | | 3/8/2012 |
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| P6.20 | Attachment Details | | 3/13/2012 |

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Revisions

WILLARD PARK
Sales Person

LOADING DOCK REPAIR

AERIAL PHOTO

Drawing Title 167460

RI Work Order Number

Drawn By

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MICHAEL BABBITT
Project Manager

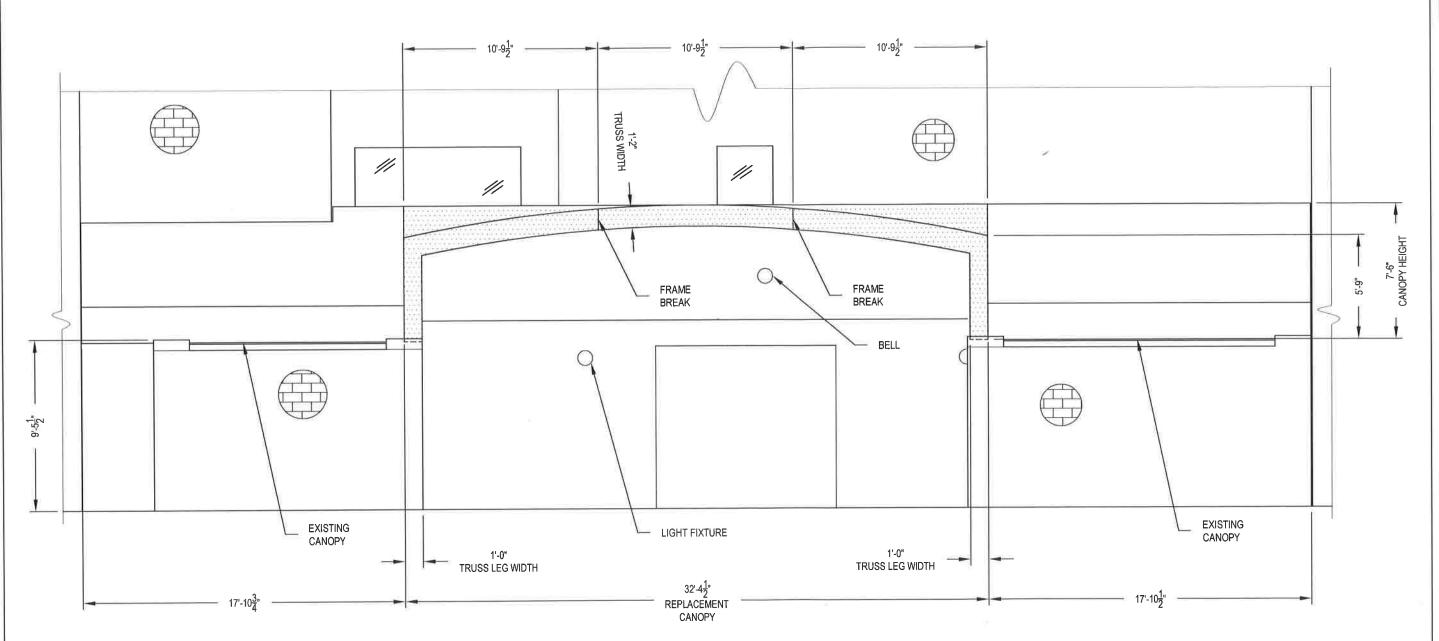
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PCD APPROVED SITE PLAN

Any proposed changes to the approved site plan, such as but not limited to added hard surfaces, HVAC units, like a minovals and accessory situatures, must be submitted to the Building Department up a revision to the building permit for review and approved by all departments prior to implementation.





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WILLARD PARK

Sales Person

LOADING DOCK REPAIR

EAST ELEVATION

Drawing Tibe

167640

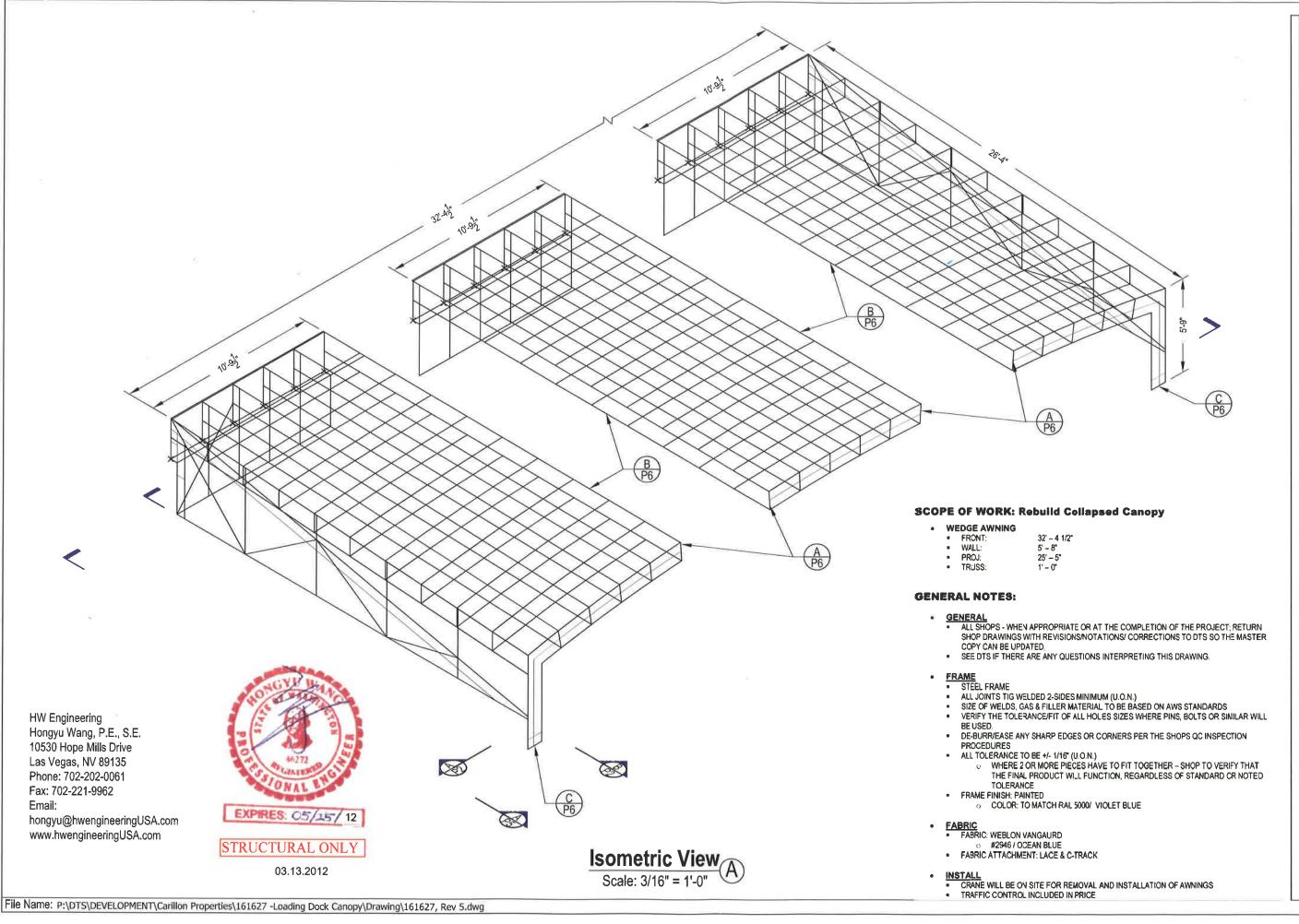
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MICHAEL BABBITT Project Manager





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WILLARD PARK

LOADING DOCK REPAIR

FULL ISOMETRIC VIEWS Drawing Title

161627

RI Work Order Number

JAN M Drawn By

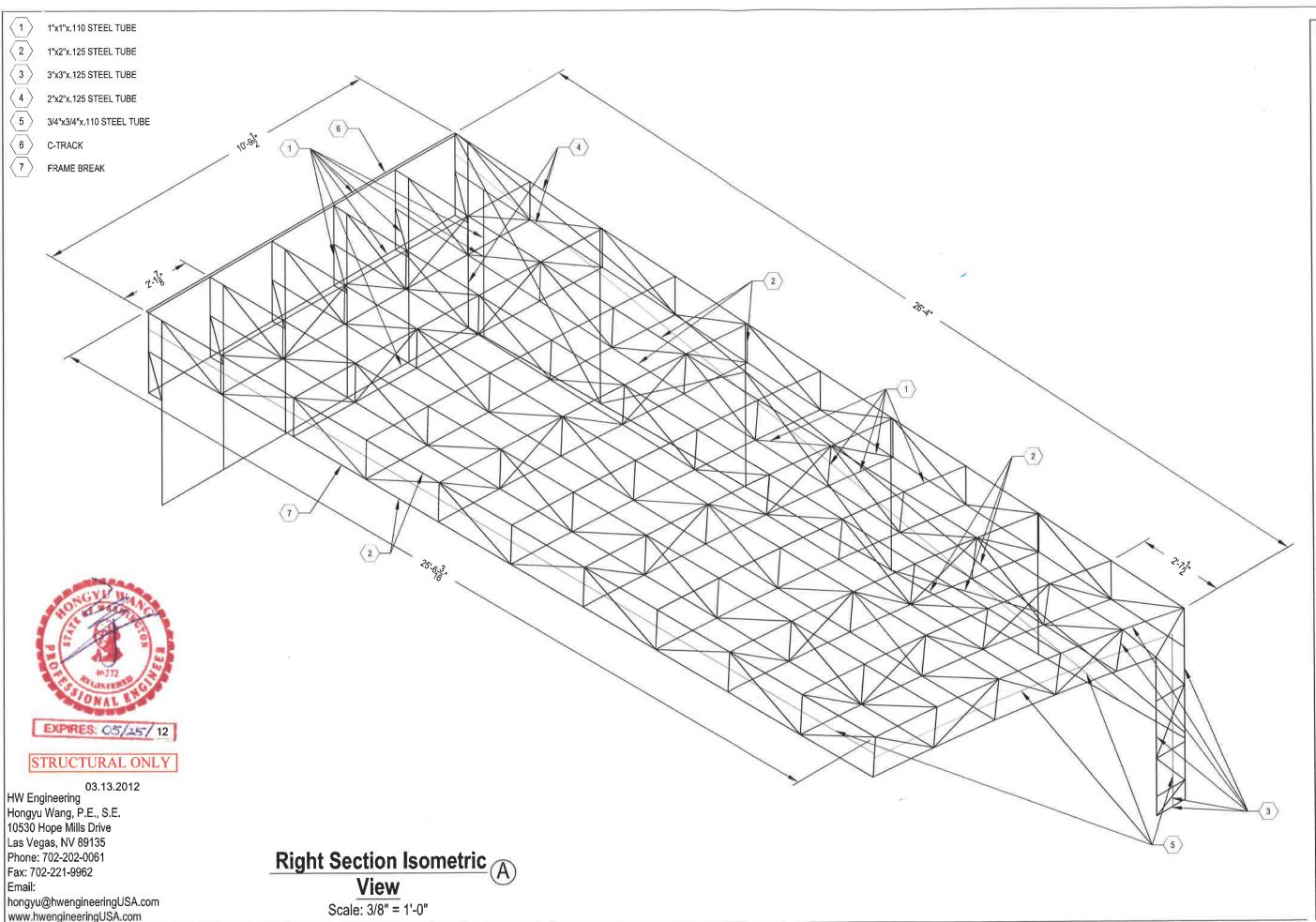
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Project Manager

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WILLARD PARK

LOADING DOCK REPAIR

Job Name

RIGHT ISO. VIEW Drawing Title

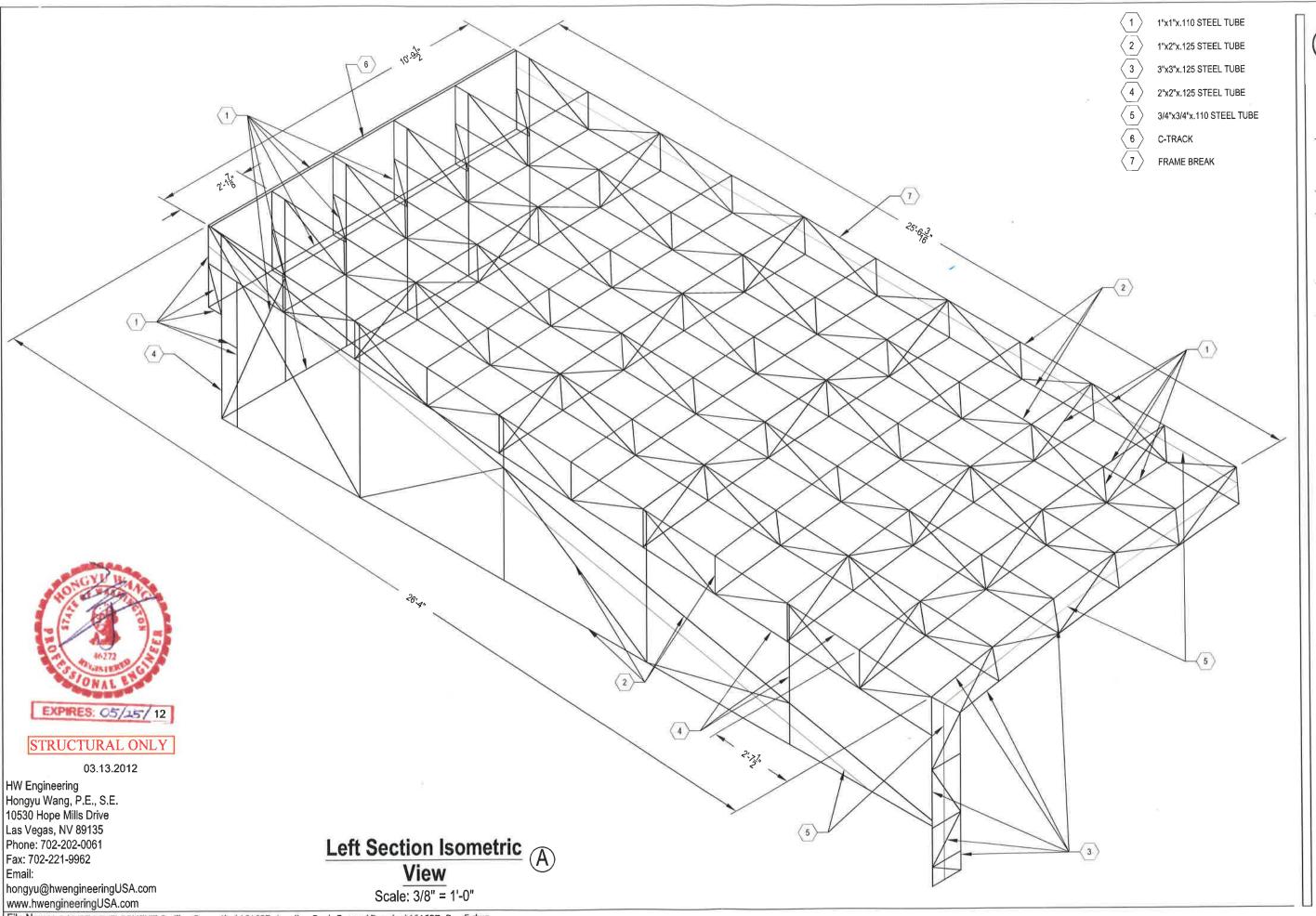
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Project Manager





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Revision

WILLARD PARK

LOADING DOCK REPAIR Job Name

LEFT ISO. VIEW
Drawing Title

161627

RI Work Order Numbe

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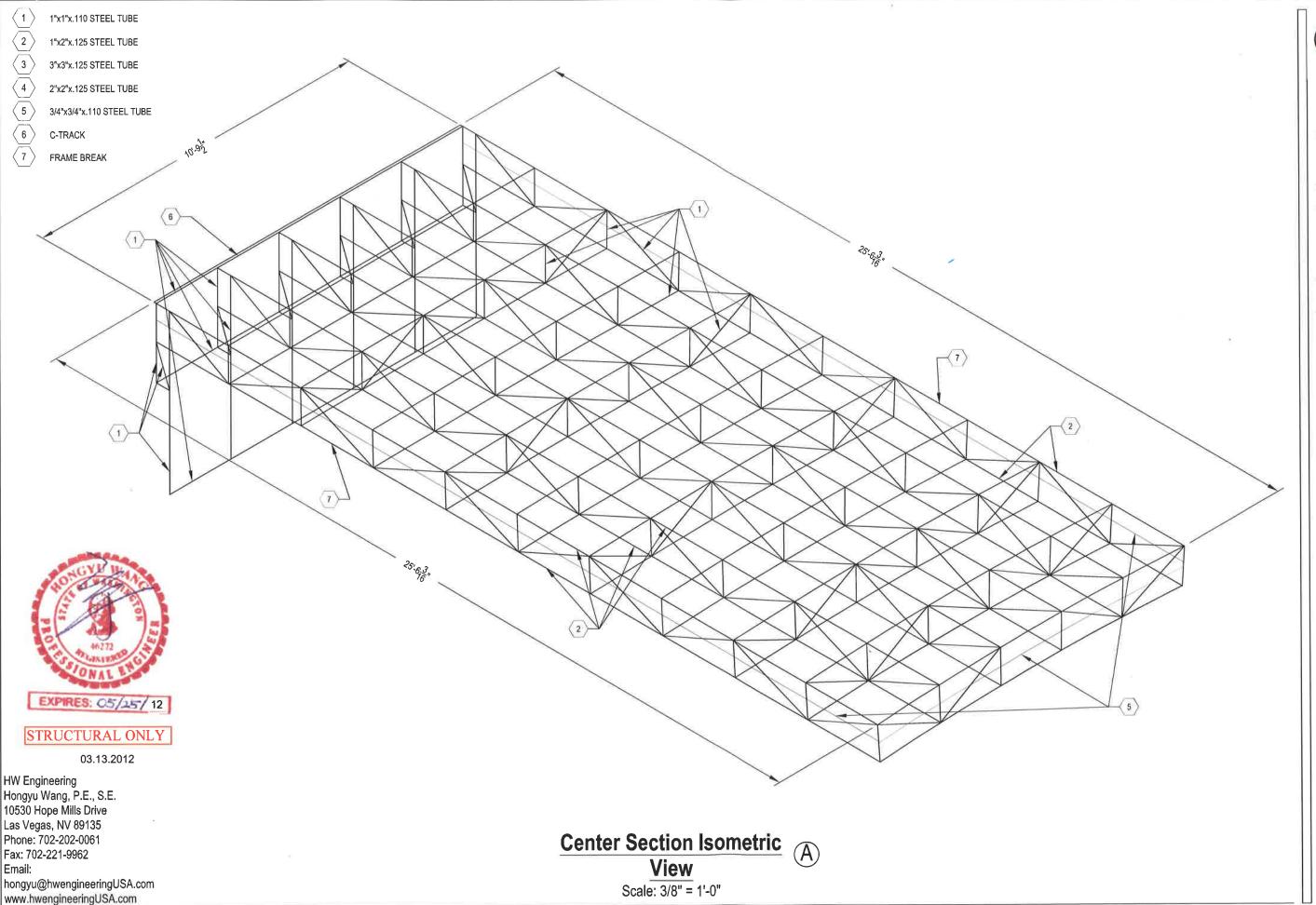
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Project Manager

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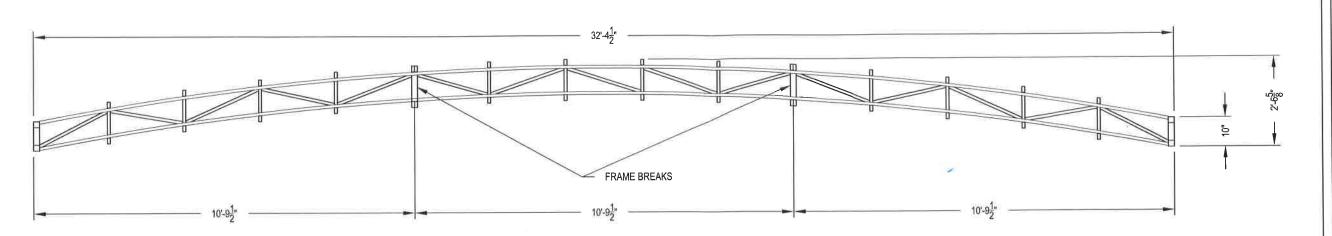
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Drawing Title

161627 RI Work Order Number

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MICHAEL BABBITT



Α

Description: Front Cross Section

QTY: -- Scale: 3/8" = 1'- 0"

Material: Steel



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Hongyu Wang, P.E., S.E.
10530 Hope Mills Drive
Las Vegas, NV 89135
Phone: 702-202-0061
Fax: 702-221-9962

Email:

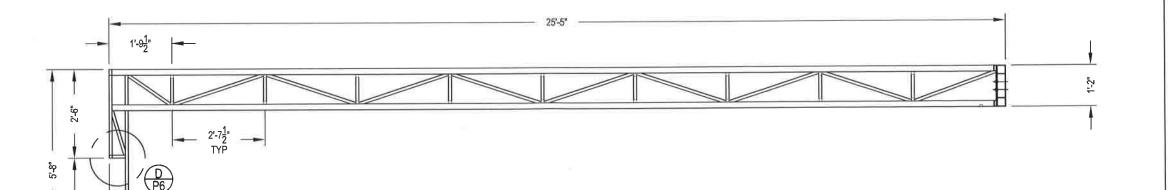
hongyu@hwengineeringUSA.com www.hwengineeringUSA.com

B Desc

Description: Side Cross Section

TY: -- Scale: 3/8" = 1'- 0" Material: Steel

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Job Name

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RI Work Order Number

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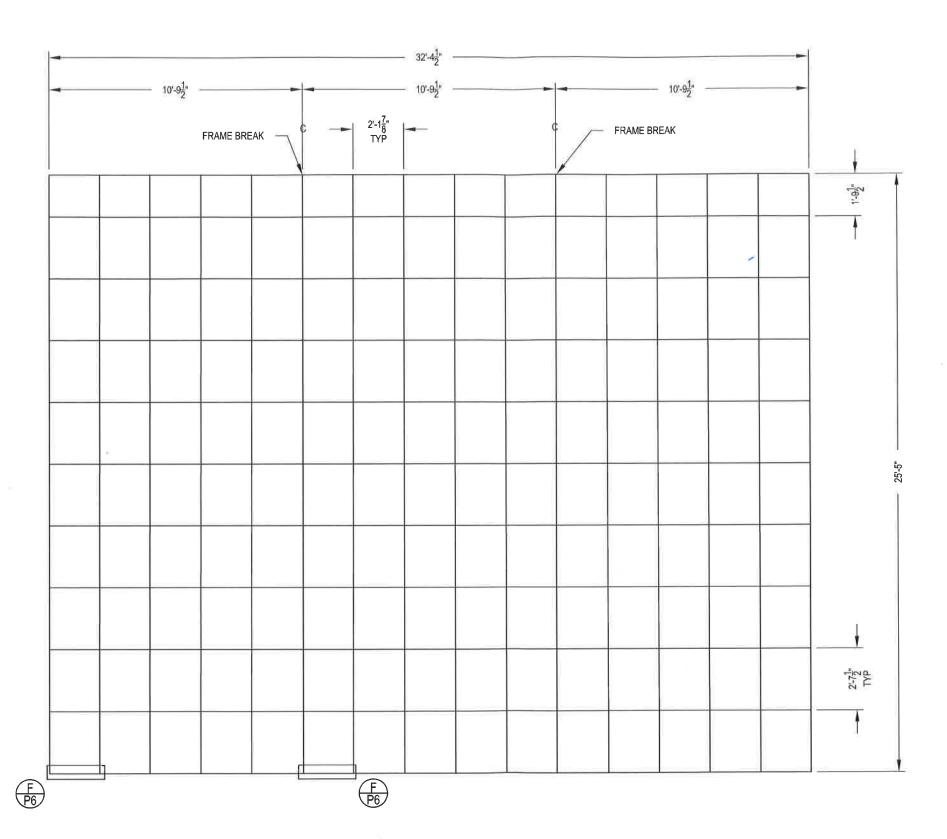
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Project Manager

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Top View Scale: 1/4" = 1'-0"



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WILLARD PARK

LOADING DOCK REPAIR

Job Name

TOP VIEW
Drawing Title

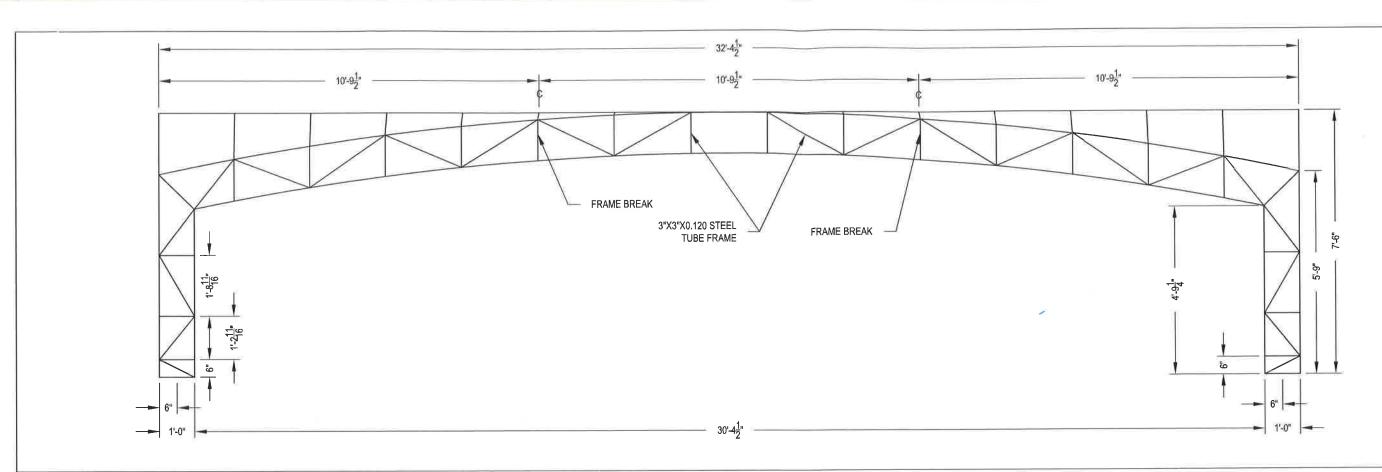
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IAN M

Drawn By

-- / --Checked By / Date

MICHAEL BABBITT
Project Manager



Description: Front View

QTY: --Scale: 3/8" = 1'- 0" Material: Steel

EXPIRES: 05/25/12

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WILLARD PARK

LOADING DOCK REPAIR

FRONT VIEW

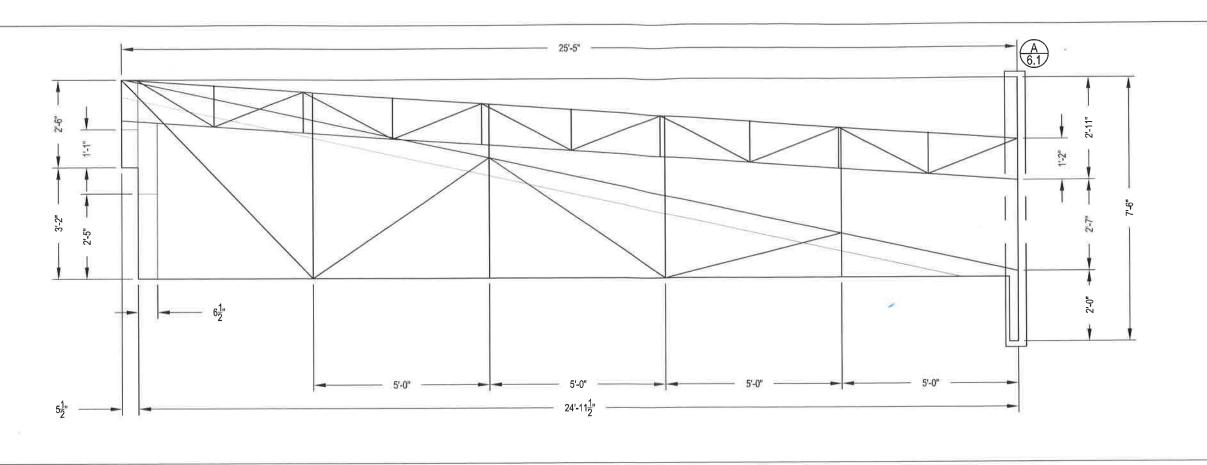
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-- / --Checked By / Date

MICHAEL BABBITT

--- / ---Issue Date / By



A

Description: Left Side View

QTY: -- Scale: 3/8" = 1'- 0" Mater

Material: Steel



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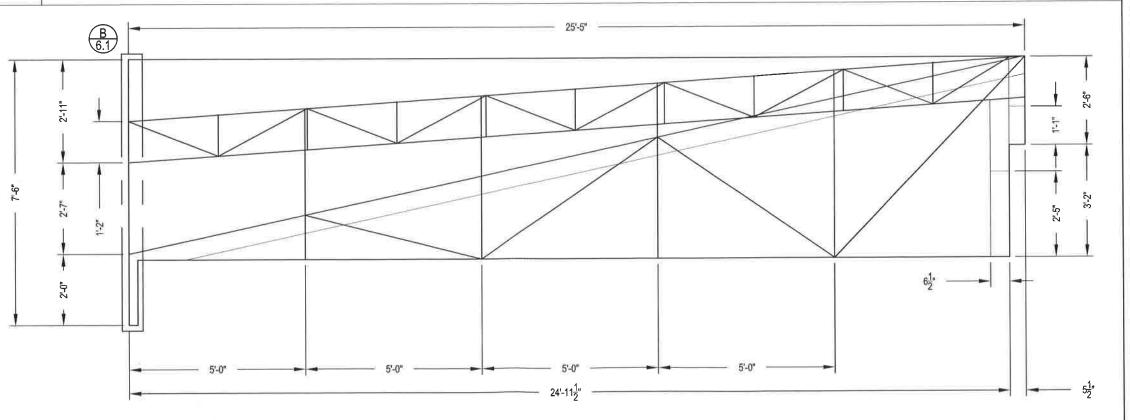
Email:

hongyu@hwengineeringUSA.com www.hwengineeringUSA.com

Description: Right Side View

QTY: -- Scale: 3/8" = 1'- 0" Material: Steel

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LOADING DOCK REPAIR

SIDE VIEWS
Drawing Title

161627 RI Work Order Number

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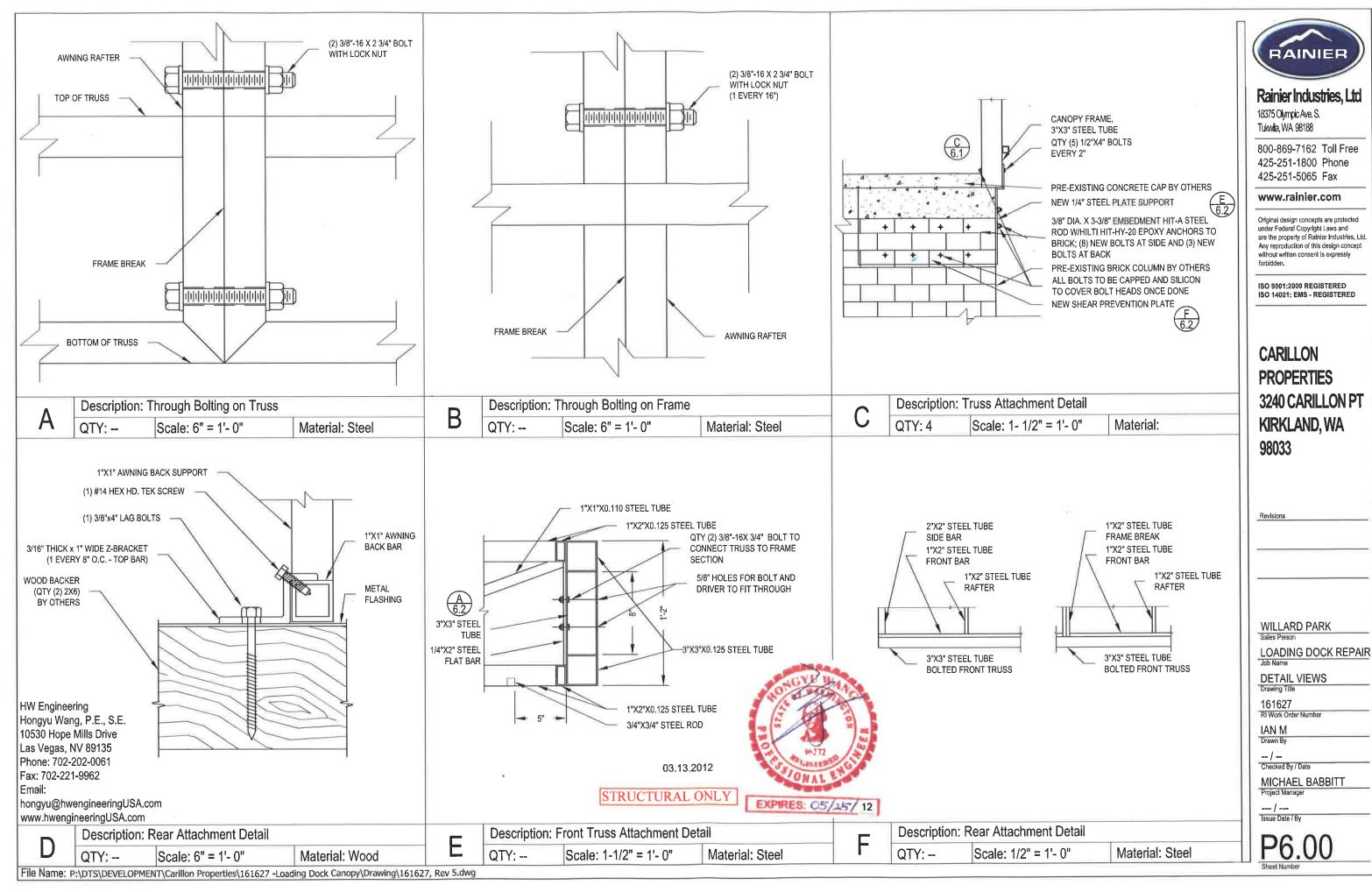
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Project Manager

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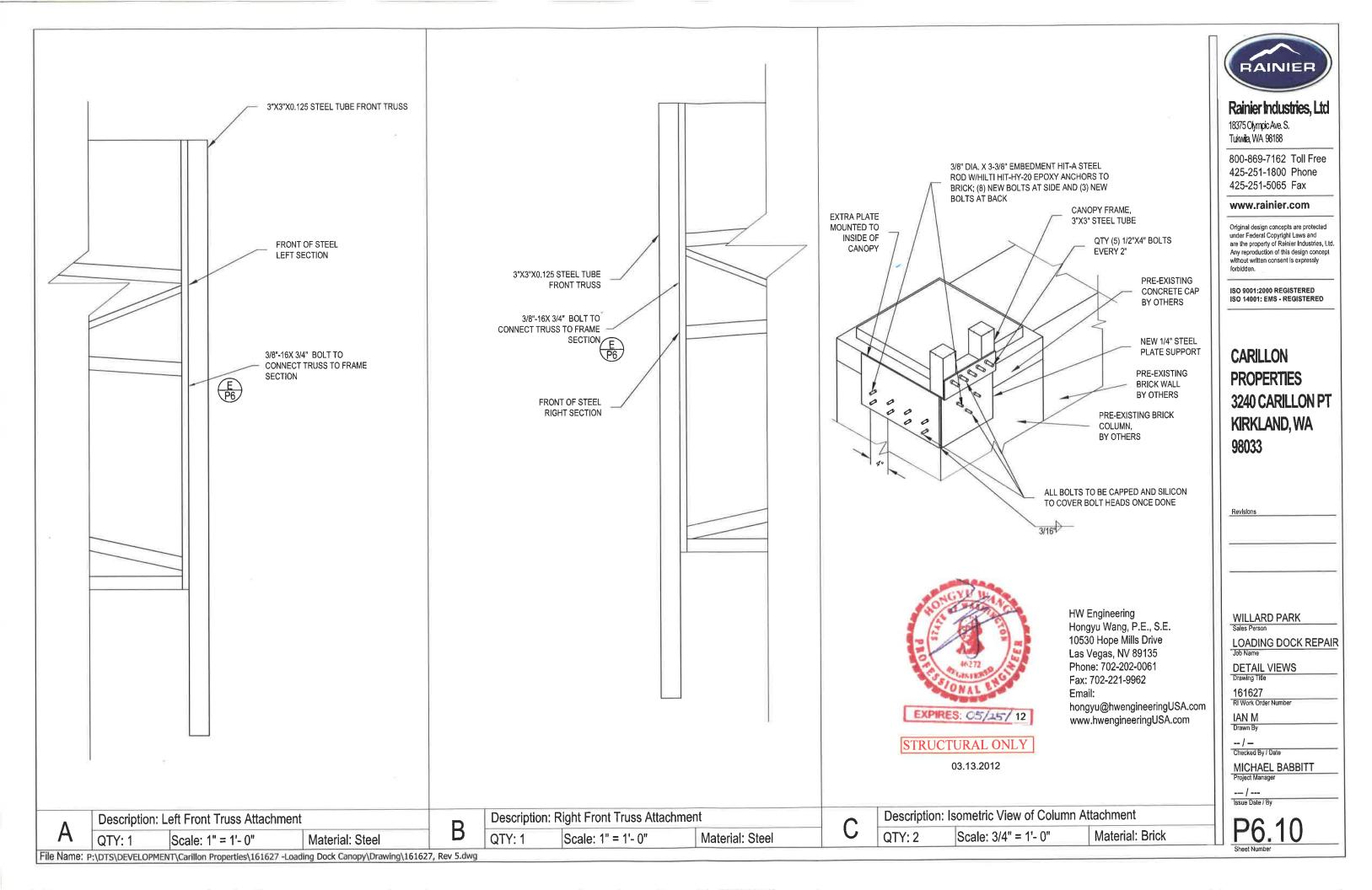
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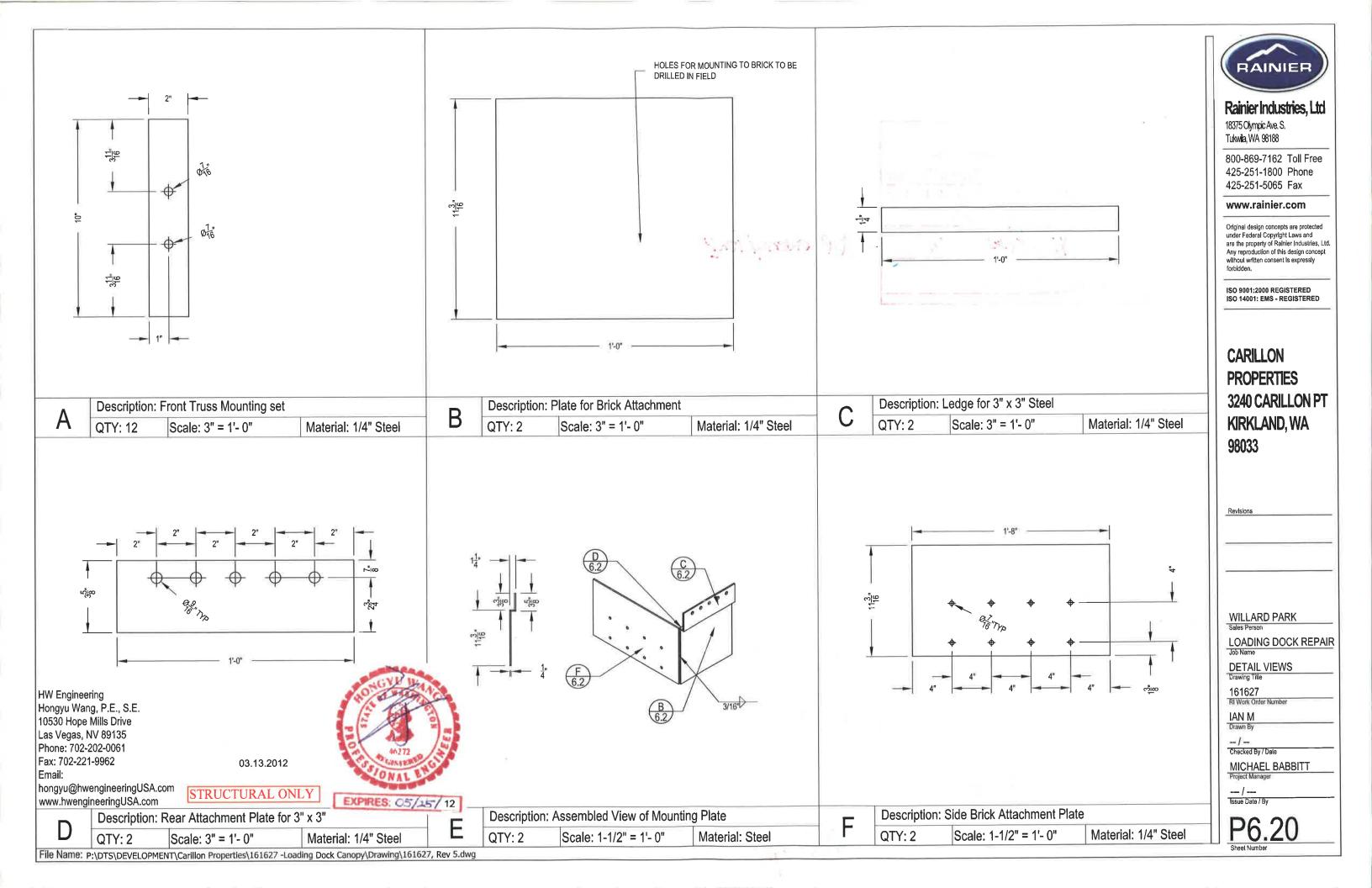
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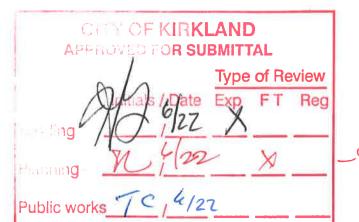


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JUL 1 7 2012

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CITY OF KIRKLAND **BUILDING DEPARTMENT**

PERMIT#

BNR12-01682

ADDRESS

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WORK CLASS

PROJECT

OWNER

CARILLON PROPERTIES W BLVD **DT SUBMITTED** 06/22/2012

REPAIR

DT APPROVED

APPROVED BY





